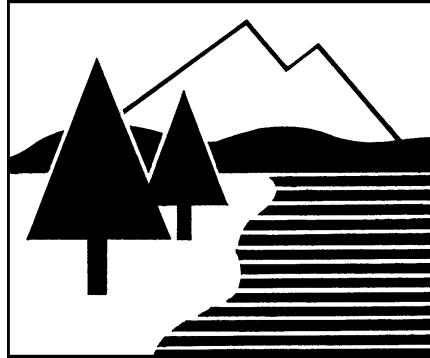


**DRAINAGE ANALYSIS**

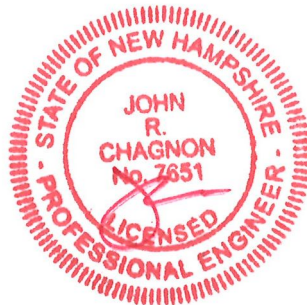
PROPOSED HOUSING DEVELOPMENT

10 LEE ROAD  
MADBURY, NH



FOR  
10 LEE ROAD, LLC

29 October 2021  
Revised 1-19-2022



**Ambit Engineering, Inc.**

Civil Engineers and Land Surveyors  
200 Griffin Road, Unit 3  
Portsmouth, NH 03801  
Phone: 603.430.9282; Fax: 603.436.2315  
E-mail: [jrc@ambitengineering.com](mailto:jrc@ambitengineering.com)  
(Ambit Job Number 3142)

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## **EXECUTIVE SUMMARY**

This drainage analysis examines the pre-development (existing) and post-development (proposed) stormwater drainage patterns for the proposed building and associated future site improvements at 10 Lee Road in Madbury, NH. The site is shown on the Town of Madbury Assessor's Tax Map 8 as Lot 9. The total size of the lot is 1,586,706 square-feet (36.4258 acres).

The development will provide for the future construction of a 6,996 square-foot building, with associated landscaping, utilities, and parking. The new buildings will be serviced by public water and electricity. Septic systems will be provided on-site. The development has the potential to increase stormwater runoff to adjacent properties, and therefore must be designed in a manner to prevent that occurrence. This will be done primarily by capturing stormwater runoff and routing it through appropriate stormwater facilities, designed to ensure that there will be no increase in peak runoff from the site as a result of this project.

The hydrologic modeling utilized for this analysis uses the "Extreme Precipitation" values for rainfall from The Northeast Regional Climate Center (Cornell University).

## **INTRODUCTION / PROJECT DESCRIPTION**

This drainage report is designed to assist the owner, planning board, contractor, regulatory reviewer, and others in understanding the impact of the proposed development project on local surface water runoff and quality. The project site is shown on the Town of Madbury, NH Assessor's Tax Map 8 as Lot 9. Bounding the site to north is NH Route 155 and a trust. Bounding the site to east and south is Town property, private residences, and an LLC. Bounding the site to the west is private residences and a Town conservation area. A vicinity map is included in the Appendix to this report.

The proposed development will add a new building and a new parking area adjacent to existing buildings on the site. This report makes some concept assumptions as to the future impervious coverage of the proposed lots, as requested by the Town.

This report includes information about the existing site and the proposed building necessary to analyze stormwater runoff and to design any required mitigation. The report includes maps of pre-development and post-development watersheds, subcatchment areas and calculations of runoff. The report will provide a narrative of the stormwater runoff and describe numerically and graphically the surface water runoff patterns for this site.

Proposed stormwater management methods will also be described, as well as erosion and sediment control practices. To fully understand the proposed site development the reader should also review a complete site plan set in addition to this report.

## **METHODOLOGY**

"Extreme Precipitation" values from The Northeast Regional Climate Center (Cornell University) have been used for modeling purposes. These values have been used in this analysis.

This report uses the US Soil Conservation Service (SCS) Method for estimating stormwater runoff. The SCS method is published in The National Engineering Handbook (NEH), Section 4 "Hydrology" and includes the Technical Release No. 20, (TR-20) "Computer Program for Project Formulation Hydrology", and Technical Release No. 55 (TR-55) "Urban Hydrology for Small Watersheds" methods. This report uses the HydroCAD version 10.0 program,

written by HydroCAD Software Solutions LLC, Chocorua, N.H., to apply these methods for the calculation of runoff and for pond modeling.

Time of Concentration (Tc) is calculated by entering measured flow path data such as flow path type, length, slope and surface characteristics into the HydroCAD program. For the purposes of this report, a minimum time of concentration of 5 minutes is used.

The storm events used for the calculations in this report are the 2-year, 10-year, and 25-year (24-hour) storms. Watershed basin boundaries have been delineated using topographic maps prepared by Ambit Engineering and field observations to confirm.

### **SITE SPECIFIC INFORMATION**

Based on the United States Department of Agriculture (USDA) Natural Resources Conservation Service (NRCS), Soil Survey of Rockingham County, New Hampshire the site is made up of eight soil types:

<b>Soil Symbol</b>	<b>Soil Name and Slopes</b>
<b>CfB</b>	Charlton fine sandy loam, 3 to 8 percent slopes
<b>CsB</b>	Charlton fine sandy loam, 3 to 8 percent slopes, very stony
<b>PdB</b>	Paxton fine sandy loam, 0 to 8 percent slopes, very stony
<b>PdD</b>	Paxton fine sandy loam, 15 to 25 percent slopes, very stony
<b>PdE</b>	Paxton very stony fine sandy loam, 25 to 60 percent slopes
<b>ScA</b>	Scantic silt loam, 0 to 3 percent slopes
<b>SuB</b>	Sutton fine sandy loam, 0 to 8 percent slopes, very stony
<b>WsB</b>	Woodbridge fine sandy loam, 0 to 8 percent slopes, very stony

All existing and proposed site development takes place on one soil type:

CfB – Charlton fine sandy loam, 3 to 8 percent slopes. The soil report qualifies the depth to the water table or restrictive feature as greater than 80 inches. The Hydrologic Soil Group is class B, with an estimated infiltrative capacity of moderately low to high (0.14 to 14.17 in/hr).

A copy of the custom soil survey for this project site is included in the Appendix to this report.

The physical characteristics of the site consist of flat (1-3%) to steep (25-60%) grades that generally slope from the north to the south of the lot. Elevations on the site range from 140 to 194 feet above sea level. The existing site is partially developed and includes two existing buildings located at the north of the lot, with an asphalt parking area. Vegetation around the developed portion of the lot consists of established grasses, shrubs, and trees. The rest of the lot is undeveloped, forested land.

According to the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) number 33017C0320E (effective date September 30, 2015), the project site is located in Zone X and is determined to be outside of the 0.2% annual chance floodplain. A copy of the FIRM map is included in the Appendix.

### **PRE-DEVELOPMENT DRAINAGE**

In the pre-development condition, the site has been analyzed as five subcatchment basins (E1, E2, E3, E4, and E5) based on localized topography and discharge location.

Subcatchment E1 contains all the proposed development area and most of the existing development, and drains to the west of the north “arm” of the lot. Subcatchment E2 contains a minor amount of developed area and drains to the east of the north arm of the lot. Subcatchment E3 drains south to Beards Creek running through the middle of the lot and then to the east. Subcatchment E4 is a small area diverted from subcatchment E3, which drains to the east. Subcatchment E5 forms the entire southern “arm” of the property, which drains to the north in the direction of Beards Creek. E5 contains some development in the form of electrical lines, but that land use is not expected to change the impervious area in the analysis, and will not be altered by the proposed development.

**Table 1: Pre-Development Watershed Basin Summary**

<b>Watershed Basin ID</b>	<b>Basin Area (SF)</b>	<b>Tc (MIN)</b>	<b>CN</b>	<b>10-Year Runoff (CFS)</b>	<b>25-Year Runoff (CFS)</b>	<b>To Design Point</b>
<b>E1</b>	295,488	19.0	60	7.75	13.94	DP1
<b>E2</b>	15,070	5.5	75	1.36	1.98	DP1
<b>E3</b>	232,162	58.3	58	2.35	4.51	DP1
<b>E4</b>	371	5.0	55	0.01	0.02	DP1
<b>E5</b>	1,032,671	24.0	68	38.00	60.48	DP1

### **POST-DEVELOPMENT DRAINAGE**

The proposed development has been designed to match the pre-development drainage patterns to the greatest extent feasible. In the post-development condition, the site has been analyzed as six major watershed basins, (P1a, P1b, P2, P3, P4, and P5). Since the design of the future houses is currently conceptual, some assumptions were made regarding the delineation of watershed basins and the determination of land usage. The delineation between Basins P2, P3, P4 and P5 correspond to approximately the same as the delineation between E2, E3, E4 and E5, respectively. Subcatchments P1a and P1b were subdivided from subcatchment P1, to represent the distinction between relatively unchanged land in P1a, and proposed development in P1b. Subcatchments P1a, P1b, and P2 are the only lots with any expected change from the proposed development.

Subcatchment P1a contains a significant increase in impervious area, and as such, will be detained in a detention pond, and released with an outlet control structure. In the event of a 100-year storm event or an outlet control structure failure, an emergency bypass was designed to release stormwater. Since all subcatchments ultimately flow to Beards Creek, the creek will serve as the considered discharge point for the entire property.

**Table 2: Post-Development Watershed Basin Summary**

Watershed Basin ID	Basin Area (SF)	Tc (MIN)	CN	10-Year Runoff (CFS)	25-Year Runoff (CFS)	Design Point
P1a	214,195	19.0	58	4.82	9.06	DP1
P1b	83,653	6.9	81	8.82	12.28	DP1
P2	13,441	5.5	66	0.83	1.32	DP1
P3	231,924	58.3	58	2.35	4.50	DP1
P4	382	5.0	55	0.01	0.02	DP1
P5	1,032,212	24.0	70	38.00	60.48	DP1

The overall impervious coverage of the area analyzed in this report for all basins **increases** from 0.704 acres (1.95%) in the pre-development condition to 1.238 acres (3.42%) in the post-development condition. Since the site represents an increase in impervious area, the project proposes the construction of a detention pond to reduce the peak flow discharge from the site.

Table 3 shows a summary of the comparison between pre-developed flows and post-developed flows for the design point in Beards Creek (DP1). The comparison considers the reduced flows as a result of infiltration.

**Table 3: Pre-Development to Post-Development Comparison**

Design Point	Q2 (CFS)		Q10 (CFS)		Q25 (CFS)		Description
	Pre	Post	Pre	Post	Pre	Post	
DP1	15.49	15.21	46.17	45.28	75.59	74.37	Beards Creek

Note that all drainage points of interest experience lower or equal peak flows.

**OFFSITE INFRASTRUCTURE CAPACITY**

There is no Town infrastructure utilized in this project in regard to storm drainage. All retention and routing to the final destination of the stormwater is done on-site.



## **EROSION AND SEDIMENT CONTROL PRACTICES**

The erosion potential for this site as it exists is moderate due to the presence of gravel areas that are highly erodible. During construction, the major potential for erosion is wind and stormwater runoff. The contractor will be required to inspect and maintain all necessary erosion control measures, as well as installing any additional measures as required. All erosion control practices shall conform to “The Stormwater Management and Erosion Control Handbook for Urban and Developing Areas in New Hampshire.” Some examples of erosion and sediment control measures to be utilized for this project during construction may include:

- Silt Soxx (or approved alternative) located at the toe of disturbed slopes
- Catch Basin Filters
- Stabilized construction entrance at access point to the site
- Temporary mulching and seeding for disturbed areas
- Spraying water over disturbed areas to minimize wind erosion

After construction, permanent stabilization will be accomplished by permanent seeding, landscaping, and surfacing the access drives and parking areas with asphalt paving and other areas with concrete walkways.

## **CONCLUSION**

The proposed development has been designed to match the pre-development drainage patterns to the greatest extent feasible. With the design of the holding pond, the post-development runoff rates are reduced to be below the pre-development runoff rates. Erosion and sediment control practices will be implemented for both the temporary condition during construction and for final stabilization after construction. Therefore, there are no negative impacts to downstream receptors or adjacent properties anticipated as a result of this project.

**REFERENCES**

1. Comprehensive Environmental Inc. and New Hampshire Department of Environmental Services. *New Hampshire Stormwater Manual (Volumes 1, 2 and 3)*, December 2008 (Revision 1.0).
2. Minnick, E.L. and H.T. Marshall. *Stormwater Management and Erosion and Sediment Control Handbook for Urban and Developing Areas in New Hampshire*, prepared by Rockingham County Conservation District, prepared for New Hampshire Department of Environmental Services, in cooperation with USDA Soil Conservation Service, August 1992.
3. HydroCAD Software Solution, LLC. *HydroCAD Stormwater Modeling System Version 10.0* copyright 2013.

**APPENDIX F**  
**INSPECTION & MAINTENANCE PLAN**



**AMBIT ENGINEERING, INC.**  
**Civil Engineers & Land Surveyors**

***INSPECTION & LONG-TERM MAINTENANCE PLAN***  
***FOR***  
**PROPOSED HOUSING DEVELOPMENT**

**10 LEE ROAD**  
**MADBURY, NH**

**Introduction**

The intent of this plan is to provide 10 Lee Road, LLC (herein referred to as “owner”) with a list of procedures that document the inspection and maintenance requirements of the stormwater management system for this development. Specifically, the detention pond and associated structures on the project site (collectively referred to as the “Stormwater Management System”). The contact information for the owner shall be kept current, and if there is a change of ownership of the property this plan must be transferred to the new owner.

The following inspection and maintenance program is necessary to keep the stormwater management system functioning properly and will help in maintaining a high quality of stormwater runoff to minimize potential environmental impacts. By following the enclosed procedures, the owner will be able to maintain the functional design of the stormwater management system and maximize its ability to remove sediment and other contaminants from site generated stormwater runoff.

**Annual Report**

The owner shall prepare an annual Inspection & Maintenance Report. The report shall include a summary of the system’s maintenance and repair by transmission of the Inspection & Maintenance Log and other information as required. A copy of the report is required to be delivered annually to the Madbury Planning Board.

***Inspection & Maintenance Checklist/Log***

The following pages contain the Stormwater Management System Inspection & Maintenance Requirements and a blank copy of the Stormwater Management System Inspection & Maintenance Log. These forms are provided to the owner as a guideline for performing the inspection and maintenance of the Stormwater Management System. This is a guideline and should be periodically reviewed for conformance with current practice and standards.

## *Stormwater Management System Components*

The Stormwater Management System is designed to mitigate both the quantity and quality of site-generated stormwater runoff. As a result, the design includes the following elements:

### *Non-Structural BMPs*

Non-Structural best management practices (BMP's) include temporary and permanent measures that typically require less labor and capital inputs and are intended to provide protection against erosion of soils. Examples of non-structural BMP's on this project include but are not limited to:

- Temporary and Permanent mulching
- Temporary and Permanent grass cover
- Shrubs and ground covers
- Miscellaneous landscape plantings
- Dust control
- Tree protection
- Topsoiling
- Sediment barriers
- Stabilized construction entrance

### *Structural BMPs*

Structural BMP's are more labor and capital-intensive structures or installations that require more specialized personnel to install. Examples on this project include but are not limited to:

- Sediment Forebay
- Detention Pond
- Outlet Control Structure

## *Inspection and Maintenance Requirements*

The following summarizes the inspection and maintenance requirements for the various BMP's that may be found on this project.

1. **Grassed areas:** During construction after each rain event of 0.5" or more during a 24-hour period, inspect grassed areas for signs of disturbance, such as erosion. If damaged areas are discovered, immediately repair the damage. Repairs may include adding new topsoil, lime, seed, fertilizer, and mulch. After construction review grassed areas after large storm events.
2. **Plantings:** Planting and landscaping (trees, shrubs) shall be monitored bi-monthly during the first year to insure viability and vigorous growth. Replace dead or dying vegetation with new stock and adjust the conditions that caused the dead or dying vegetation. During dryer times of the year, provide weekly watering or irrigation during the establishment period of the first year. Make the necessary adjustments to ensure long-term health of the vegetated covers, i.e., provide more

permanent mulch or compost or other means of protection.

3. **Storm Drain and Catch Basin Inlets/Outlets:** Monitor drain inlets and outlet aprons for excessive accumulation of sediments or missing stone/riprap. Remove sediments as required to maintain filtering capabilities of the stone—replace missing riprap.
4. **Detention Pond:** After installation of the detention pond, perform the following inspections on an annual basis:
  - a. Monitor for excessive or concentrated accumulations of debris, or excessive erosion below the various pipe inlets. Remove debris as required and replace or augment inlet fabric strips.
  - b. Monitor the outfall structure for problems with uneven flow or clogged pipes. Repair or remove clogs as required.
  - c. Monitor vegetation on pond and replace dead or dying vegetation as required.
  - d. Monitor rodent screens and repair or replace as required.
  - e. Monitor side slopes of ponds for damage or erosion—repair, as necessary.

### **Pollution Prevention**

The following pollution prevention activities shall be undertaken to minimize potential impacts on stormwater runoff quality. The Contractor is responsible for all activities during construction. The Owner is responsible thereafter.

#### ***Spill Procedures***

Any discharge of waste oil or other pollutant shall be reported immediately to the New Hampshire Department of Environmental Services (NHDES). The Contractor/Owner will be responsible for any incident of groundwater contamination resulting from the improper discharge of pollutants to the stormwater system and may be required by NHDES to remediate incidents that may impact groundwater quality. If the property ownership is transferred, the new owner will be informed of the legal responsibilities associated with operation of the stormwater system, as indicated above.

#### ***Sanitary Facilities***

Sanitary facilities shall be provided during all phases of construction.

#### ***Material Storage***

No on site trash facility is provided. The customers are required to remove trash from the site. Hazardous material storage is prohibited.

#### ***Material Disposal***

All waste material, trash, sediment, and debris generated during construction shall be removed from the site and disposed of in accordance with applicable local, state, and federal guidelines and regulations. Removed sediments shall be if necessary dewatered prior to disposal.

### **Snow & Ice Management for Standard Asphalt and Walkways**

Snow storage will be located such that no direct untreated discharges are possible to receiving waters from the storage site.

### **Invasive Species**

Monitor the Stormwater Management System for signs of invasive species growth. If caught early, their eradication is much easier. The most likely places where invasions start is in wetter, disturbed soils or detention ponds. Species such as phragmites and purple loose-strife are common invaders in these wetter areas. If they are found, the owner shall refer to the fact-sheet created by the University of New Hampshire Cooperative Extension or contact a wetlands scientist with experience in invasive species control to implement a plan of action for eradication. Measures that do not require the application of chemical herbicides should be the first line of defense.





# New Well Location Approval for Non-Community Public Water Systems

NHDES Drinking Water & Groundwater Bureau  
Design Review



RSA/Rule: 485; Env-Dw 406

**Directions:**

For non-community water systems only – Please complete this form to seek approval to install a new well that will serve a public water system.

If you have any questions, please contact the DWGB at (603) 271-2513.

**Definitions:**

**Non-Community Water System** – RSA 485:1-a, XV and X; Env-Dw 103.37

A public water system of piped water for human consumption that is not a community water system, with at least 15 service connections or regularly serves an average of at least 25 individuals daily at least 60 days out of the year.

**Non-transient, Non-community (NTNC) Water System** – RSA 485:1-a, XI; Env-Dw 103.38

A non-community water system that serves the same 25 people, or more, over 6 months per year. Examples: child care centers, schools and workplaces.

**Transient, Non-community (TNC) Water System** – Env-Dw 103.62

A non-community water system that serves at least 25 persons in a transitory setting for more than 60 days each year. Examples: Restaurants, campgrounds, hotels, and places that serve coffee or other beverage service.

Application Date: <u>1-17-2022</u>		Design Review Number (NHDES use only): _____	
PWS ID Number: <u>1456020</u> <small>(for existing systems)</small>			
<u>10 LEE ROAD</u>	<u>MAP 8 LOT 9</u>		
Name of Project	Map & Lot Number		
<u>10 LEE ROAD</u>	<u>MADBURY</u>		
Location of well (street address)	Location of well (city/town)		
Check water system type: <input checked="" type="checkbox"/> Non-Transient, Non-Community (NTNC) <input type="checkbox"/> Transient, Non-Community (TNC)			
<b>1. Property Owner Information</b>			
<u>10 LEE ROAD, LLC</u>		<u>JSPETROVITSIS@GMAIL.COM</u>	
Name of Legal Property Owner		Email address	
<u>JAMES PETROVITSIS</u>		<u>603-828-2735</u>	
Contact Name		Telephone Number	
<u>1 BAYSIDE ROAD, BOX 4</u>		_____	
Mailing Address		Fax Number	
<u>GREENLAND</u>		<u>NH</u>	<u>03840</u>
City/Town		State	ZIP Code + 4
<b>2. Agent Information</b> (if someone has been hired to assist the owner with an application, please provide the contact information. This contact will be used as the primary contact during approval.)			
<u>AMBIT ENGINEERING, INC</u>		<u>JRC@AMBIT ENGINEERING.COM</u>	
Agent Company, if applicable		Email address	
<u>JOHN CHAGNON, PE</u>		<u>603-430-9282</u>	
Contact Name		Telephone Number	
<u>200 GRIFFIN ROAD, UNIT 3</u>		<u>603-436-2315</u>	
Mailing Address		Fax Number	
<u>PORTSMOUTH</u>		<u>NH</u>	<u>03801-7114</u>
City/Town		State	ZIP Code + 4



**3. Justification**

State if there is an existing public water supply nearby and if so, why you are not able to connect. Also state why the new well(s) is needed. *Examples: 1) Systems expansion from 40 people to 100 people. 2) Declining yield from existing source 3) New facility that needs water*

*No municipal supply nearby. Existing private well. Expansion of demand; existing well radius not protected.*

**4. Description of the service provided or to be provided, with respect to water use.**

Include information about irrigation and fire flows, if applicable. *For example: The new well will serve a campground with 80 campsites or a restaurant with 100 seats.*

The new well will service 19 Apartments; DES design flow (peak) of 9,300 GPD

**5. Design Flow – Env-Dw 406.08**

Calculate Design Flow using Table 406-1: 9,300 gallons per day (gpd)

Enter supporting calculations below including irrigation and fire flow needs, if applicable:

62 Bedrooms X 150 GPD / Bedroom = 9,300 GPD

**6. Number of Equivalent Units – Env-Dw 406.06**

\_\_\_\_\_ gpd X  $\frac{1 \text{ eq. unit}}{300 \text{ gpd}}$  = \_\_\_\_\_ eq. unit(s) *(round to nearest whole number)*

*Note: No fee is needed for Well Locations Approvals. A fee is required for Final Design Approvals.*

**7. Please list any other wells or water supplies on the property and their uses.**

None

**8. Sanitary Protective Area – Env-Dw 406.12**

Determine the Sanitary Protective Area (SPA) using the calculated design flow from Section 5 and Table 406-2

\*For NCWS systems, use the design flow from Section 5 as the permitted production volume. For NTNC systems, multiply the design flow from Section 5 by 1.5 to calculate the permitted production volume.

**SPA = 150 (175) feet**

The SPA is FREE of (check all applicable boxes):

- Wastewater systems (septic tanks, grease traps, effluent disposal areas/leach fields);
- Fertilization areas;
- Dumpsters;
- Detention ponds, infiltration systems, storm water treatment systems; and
- Fuel of other hazardous chemicals such as: oil, gasoline, underground propane and natural gas tanks, etc.

List any other use in the SPA: Gravel drive to well; pump house

*If any of these are located in the SPA, attach a waiver request.- Env-Dw 202.03*

Is the well at least 50 feet from surface waters, wetlands, & natural drainage ways? **Yes** (yes/no)

Is the well at least 50 feet from the edge of the right-of-way, driveways, & parking areas? **Yes** (yes/no)

Is the wellhead above the 100-year floodplain elevations? **Yes** (yes/no)

Is the SPA within the owner’s property? **Yes** (yes/no)

Is the wellhead outside the leach field nitrate set-back areas? **Yes** (yes/no)

*If any of these answers are “no”, attach a waiver request. - Env-Dw 202.03*

**9. Additional System Information**

Do you have pressure storage? **Yes** (yes/no) If yes, how many gallons of storage? **TBD**

Do you have atmospheric storage tanks? **TBD** (yes/no) If yes, how many gallons of storage? **TBD**

**10. Application Check List - include the following with this application form (check all that apply):**

- Waivers Request – Env-Dw 202
- Well Location Plan with the following:
  - North Arrow
  - Scale
  - Property boundaries
  - Lot numbers
  - Name of adjacent owners
  - Contours, if available
  - Nitrate setbacks
  - Surface waters, wetlands, drainage ways
  - 100-year floodplain elevation
  - Wellhead location
  - SPA, with the radius noted
  - 50’ radius (common setback)
  - Wastewater disposal areas – existing and proposed
  - Buildings, roads, right-of-way, driveways, and parking areas

**11. Signature Required:**

John Chagnon  
 Signature of owner or owner’s agent  
John Chagnon, PE  
 Print name of owner or owner’s agent

1-17-2022  
 Date

**Return completed form by mail, fax, or email to:**

NHDES Drinking Water & Groundwater Bureau  
 PO Box 95, Concord, NH 03302-0095  
 Fax: (603) 271-5171  
 Email: [dwgbinfo@des.nh.gov](mailto:dwgbinfo@des.nh.gov)